

ETABS v23.3.2 Release Notes

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Notice Date: 21-September-2026

This document lists changes made to ETABS since v23.3.1, released 02-July-2026. Items marked with an asterisk (*) in the first column are more significant.

Installation and Licensing

Enhancements Implemented

*	Ticket	Description
	12420	The version number has been changed to v23.3.2 for a new minor release.

Analysis Incidents Resolved

*	Ticket	Description
*	12203	An incident was resolved where long running load cases that updated the analysis monitor form very frequently sometimes caused ETABS to become unresponsive.
	12262	An incident was resolved in which running the analysis on certain models failed with the warning "Error in performing miRunAnalysis," and the analysis did not complete. This was an issue with v23.3.0 and v23.3.1 only.
*	12329	An incident was resolved where an "Error in performing miRunAnalysis" error could occur when running analysis after a load case was added, copied, or modified.
	12333	An incident was resolved where, when area springs are assigned to a floor object that is meshed using rectangular meshing (Analyze menu>Automatic Mesh Settings for Floors) and the floor object is added using the "Add Structure" operation in a staged construction load case, the areas springs may be missing from the staged construction add operation when the analysis is run for the first time after the automatic floor mesh is formed. The issue does not occur in subsequent analysis runs.
	12388	An incident was resolved where frame hinges in models set to model hinges as separate link elements (Analyze menu>Analysis Model for Nonlinear Hinges) may resist larger forces than expected in nonlinear load cases which set the Geometric Nonlinearity Option to "P-Delta plus Large Displacements". The effect was proportional to the magnitude of the displacements of the hinge, particularly if there were large rigid body rotations affecting the hinge. This issue only occurs for Nonlinear Static, Staged Construction, and Nonlinear Direct-Integration Time-History load cases and only occurred in ETABS v22.7.0 - ETABS v23.3.1. This issue does not affect load cases with the Geometric Nonlinearity Option set to "None" or "P-Delta" and does not affect models that are set to model hinges within the frame elements.
*	12411	An incident was resolved where running an analysis could report "Error Cleaning Group Arrays - Error No. 91" and stop. In affected models the program could also close unexpectedly when load case data was opened, and results could be discarded after a run.
*	12412	An incident was resolved where a user would sometimes receive an error when cancelling an in-progress analysis before any load cases were completed then performing an action that attempted to generate the element model, such as running a subsequent analysis.

Database Tables Incidents Resolved

*	Ticket	Description
	12265	An incident was resolved where double-clicking on any table column header divider was terminating the program. This error was introduced in v23.3.0/v23.3.1.

Design – Composite Column Incidents Resolved

*	Ticket	Description
	12238	An incident has been resolved for the IS 11384:2022 and Eurocode 4-2004 composite column designs in which the message for design failure was not shown in the design report.

Design – Concrete Frame Incidents Resolved

*	Ticket	Description
	12316	An incident was resolved for ACI 318-19 and ACI 318-25 code where "Identify All Failures" option was unable to show failure due to shear interaction ratio exceeding 1.5. The failure message was however shown in the column design summary report.
	12320	An incident was resolved where Section Designer (SD) column sections with reinforcement set to be designed were internally reset to reinforcement being checked when model file was imported from a text file.

*	Ticket	Description
	12407	An incident has been resolved for the TCVN 5574:2018 concrete frame design in which the value of Rsw has been corrected to be taken as 0.7Rs,n. Previously, it was taken as 0.7Rs.

Design – Shear Wall

Incidents Resolved

*	Ticket	Description
	12314	An incident was resolved for IS 456:2000 shear wall concrete capacity was reported as negative and shear design reported large area of shear reinforcement. This condition occurs where Hw/Lw was greater than 3 in v23.3.0 and v23.3.1. Any previous versions were not affected.
	12433	An incident was resolved for CSA A23.3-19 wall design code where there was a discrepancy between the displayed D/C ratios vs right clicking on wall report for conventional pier.

Design – Steel Frame

Incidents Resolved

*	Ticket	Description
	12378	An incident has been resolved for the CSA S16-24, CSA S16-19, and CSA S16-14 steel frame designs in which the moment resistance (Mr22) for bending about minor (Y-axis) of the angle section considering lateral-torsional buckling was computed incorrectly by using plastic section modulus. It is now corrected by using the elastic section modulus. Section 3.7.3.7 in the design manuals is also corrected. In addition, the moment resistance (Mr33) for bending about the major (X-axis) of angle sections considering local buckling is calculated using section modulus determined about principal axes.

Loading

Incidents Resolved

*	Ticket	Description
	12369	An incident was resolved for NBCC 2025 auto wind load case where multi-step load was unable to run.
	12422	An incident was resolved for KDS 41 12 00:2022 auto wind load case where wind loads were not calculated when model does not have any structure at the topmost level.

Results Display and Output

Incidents Resolved

*	Ticket	Description
	12402	An incident was resolved where drawn Section Cuts (Draw menu > Draw Section Cut) and Section Cuts defined by Quadrilateral Cutting Planes (Define menu > Section Cuts) may report incorrect forces and moments when cutting wall objects that are assigned with Vertical Wall Hinges (Assign menu > Shell > Wall Hinge). This was a reporting issue with section cuts results only, and did not affect other analysis results. Wall objects assigned with General Wall Hinges were not affected by this issue.

Structural Model

Incidents Resolved

*	Ticket	Description
	12277	An incident was resolved where fiber hinges assigned to frames that have a long and thin rectangular concrete section property with very little confinement rebar and used a Mander-type concrete material may produce an error, preventing analysis from being run.
	12408	An incident was resolved where enabling the "Use Hinge Length Overwrite Instead, if Available" checkbox in the Auto Subdivide groupbox of the "Frame Assignment - Hinge Overwrites" form (Assign menu > Frame > Hinge Overwrites) did not apply the hinge length overwrite to the hinge subdivide length as intended. This issue only affected ETABS v23.3.0 and v23.3.1.

ETABS v23.3.1 Release Notes

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Notice Date: 01-July-2026

This document lists changes made to ETABS since v23.3.0, released 24-June-2026. Items marked with an asterisk (*) in the first column are more significant.

Internal

Enhancements Implemented

*	Ticket	Description
	12226	The version number has been changed to v23.3.1 for a new minor release.

Design – Composite Beam

Incidents Resolved

*	Ticket	Description
	12207	Two related incidents affecting composite beam design overwrites were resolved. 1.) When several composite beams with different specified types of degree of composite action ("non-composite w.o. studs", "non-composite w. studs", "composite as required", or "force composite") were selected and the Composite Beam Overwrites form was activated, attempting to change the type of degree of composite action to a new single type caused an abnormal program termination. This could be avoided by changing that type a few beams at a time. 2.) Interactive database editing of composite beam overwrites triggered a number of spurious warnings when the vibrations criterion of some of the beams was "None", but otherwise worked correctly. This affected all versions of ETABS supporting interactive database editing.

Structural Model

Incidents Resolved

*	Ticket	Description
	12225	An incident was resolved where the model check would sometimes show warnings if checked prior to generating the analysis model.

User Interface

Incidents Resolved

*	Ticket	Description
*	12213	An incident has been resolved where the name of a newly created Soil Profile (Define > Spring Properties > Soil Profiles) was not being saved correctly. This could sometimes cause the program to terminate unexpectedly when using the soil profile in subsequent operations, such as creating additional soil profiles or assigning the profile to spring properties. Soil Profile names are now saved correctly, preventing these unexpected program terminations. This issue only affects v23.3.0.
	12218	An issue has been resolved where some defined pier/spandrel labels were not saved when twenty or more labels were created in a single operation. This issue was introduced in version 23.3.0 with the addition of the ability to assign colors to pier and spandrel labels. Versions prior to 23.3.0 are not affected by this issue.
*	12221	An incident was resolved where an abnormal condition would occur when opening the frame Assign Hinge form. This issue only affects v23.3.0.
*	12224	An incident was resolved where after the steel design is completed using the Chinese design code, clicking Save causes the error message "Error saving .EDB file. File may be incomplete. Object reference not set to an instance of an object."

ETABS v23.3.0 Release Notes

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Notice Date: 23-June-2026

This document lists changes made to ETABS since v23.2.0, released 24-February-2026. Items marked with an asterisk (*) in the first column are more significant.

API

Enhancements Implemented

*	Ticket	Description
	12108	An enhancement has been implemented to support Application Programming Interface (API) clients targeting .NET 10 when accessing the API via COM.
	12164	A new method, SapModel.RespCombo.AddDesignDefaultCombos_1, was added to allow users to add the default design load combinations for composite beam design and composite column design.

Database Tables

Enhancements Implemented

*	Ticket	Description
	11992	A new output database table "Story Inertial Forces" (Display menu > Show Tables) was added to output the inertial forces at each story level for modal and response spectrum load cases. This table is a direct way for users to obtain inertial forces for response spectrum which previously had to be calculated using section cuts.

Design – Composite Beam

Enhancements Implemented

*	Ticket	Description
	12160	An enhancement to composite beam design per all design codes has been implemented. A new table is now included in the output of the composite beams which do not have enough room to place enough shear studs to develop the code mandated minimum percentage of composite action between the location of the maximum positive bending moment and the nearest location of zero bending moment for some - although not all - design load combinations. This table lists the locations of the maximum positive bending moment and the nearest locations of zero bending moment, along with breakdowns of the total force developed by the shear studs in-between, for two load combinations: the most restrictive load combination, and the load combination which controls the composite design of the beam.

Design – Composite Shear Wall

Enhancements Implemented

*	Ticket	Description
	11870	A change was made implementing AISC 360-22 Table I1.1b compactness classification for composite coupling beam flexural design. Noncompact and slender sections now use the applicable AISC 360-22 Section I3.4b flexural strength reductions instead of always using plastic moment strength.

Design – Concrete Frame

Enhancements Implemented

*	Ticket	Description
	12039	A change has been made to concrete frame design code "Chinese 2010" in which the beam/column capacity ratio is no longer reported. In Chinese seismic design code (GB 50011-2010), the “strong-column-weak-beam” mechanism is guaranteed through internal force amplification rather than direct joint flexural capacity ratio check as specified in other codes. For frame structures, the design moments at column ends are amplified by a factor which is MMF in ETABS. This indirect adjustment ensures columns will yield after beams in an earthquake. So, the joint flexural capacity ratio check is no longer necessary for Chinese code.

Design – Slab

Enhancements Implemented

*	Ticket	Description
*	10959	An enhancement has been made to add concrete and PT slab design for CSA A23.3:2024 code.
	12057	An enhancement was made to Eurocode 2:2004 slab design which now performs the punching shear check at the column perimeter, as required by Section 6.4.3(2)(a).

Design – Steel Frame

Enhancements Implemented

*	Ticket	Description
	12106	An enhancement to the design of steel joists was implemented. Beams can now be assigned the joist design procedure when there are no adjacent area objects, or when none of the adjacent area objects have a deck section property.
	12168	A change has been made in steel frame deflection report where Dead load deflection now reports the deflection due to Dead type load only and Super DL are excluded from the Dead load deflection. Also, no pattern loading has been applied for live load for deflection reporting.

Drafting and Editing

Enhancements Implemented

*	Ticket	Description
	11753	An enhancement to the “Quick Draw Floor/Wall (Plan, Elev, 3D)” command was implemented. It now lets you draw a floor object around the perimeter of the entire building with a single click on any edge beam.

External Import and Export

Enhancements Implemented

*	Ticket	Description
*	12022	An enhancement to export model tables to SQLite format has been implemented.

Internal

Enhancements Implemented

*	Ticket	Description
	11866	The version number has been changed to v23.3.0 for a new minor release.

Loading

Enhancements Implemented

*	Ticket	Description
*	11934	An enhancement was made to add KDS 41 12 00:2022 auto wind load. This enhancement includes the automated computation of Gust factor for rigid and flexible structure based on the governing mode shape. Users also have option to include the Cross wind and Torsional wind effects (if applicable).
	12172	A change was made in auto seismic lateral load and response spectrum function to update the code name to "KDS 41 17 00:2022" from "KDS 41 17 00:2024" to reflect the correct publication of this standard.

Results Display and Output

Enhancements Implemented

*	Ticket	Description
	11991	The program behavior has been changed for Section Cuts defined by Group (Define menu > Section Cuts) which include an auto-meshed area object and all of the joints which define the area object. In this situation, the section cut will now include all of the internal elements and program-created joints associated with that auto-meshed area object. This behavior change only affects cases where a section cut was used to compute joint forces internal to the area object, such as inertial or applied acceleration forces. This does not affect the behavior of section cuts which include an area object and some, but not all, of its joints or section cuts defined by a cutting plane.

Structural Model

Enhancements Implemented

*	Ticket	Description
	11962	An enhancement has been made to speed up the loading calculations while generating element model.

User Interface

Enhancements Implemented

*	Ticket	Description
	2244	An enhancement has been made to sort the combinations/load cases alphabetically in all applicable list boxes in the program.
	11907	An enhancement was added to allow display by color of pier and spandrel labels.

Analysis Incidents Resolved

*	Ticket	Description
	11781	An incident was resolved where changing the diaphragm type from "Rigid" to "Semi-Rigid" while auto wind load was enabled caused the analysis to fail with the message "Error in performing miRunAnalysis." This resulted in the analysis being aborted. Previously, users had to unlock the model and rerun the analysis to complete the process successfully. This behavior has now been corrected to ensure the analysis runs successfully on the first attempt.
*	12030	An incident was resolved where staged construction load cases adding auto-meshed area objects with shell element edge release assignments failed to run.

API Incidents Resolved

*	Ticket	Description
	11665	An issue was resolved where the API functions cAnalysisResults.ModalParticipatingMassRatios and cAnalysisResults.ModalParticipationFactors always returned zero values for the output parameter StepNum instead of the mode number. No results are affected.
	12183	An incident was resolved in the API where editing model data via the DatabaseTables API methods could skip setting/updating certain fields of certain tables due to a versioning problem. The model and its results would be consistent with the values displayed in the user interface and the database tables within the program.

Data Files Incidents Resolved

*	Ticket	Description
	11945	Several incidents affecting custom joist design were resolved. 1.) Custom joist properties were not imported from .e2k files. 2.) Depending on the number of load combinations selected for joist design, custom joist design did not always show results in which case right-clicking on a custom joist beam to interactively design it would cause an abnormal program termination 3.) Custom joist sections were not exported to the databases.

Database Tables Incidents Resolved

*	Ticket	Description
	11735	An incident was resolved in the Concrete Beam Shear Envelope table for Eurocode 2-2004 and Eurocode 2-2023 where no values were reported in the "Asl Top" and "Asl Bottom" columns.
	12054	An incident was resolved where using a Section Designer section for a curved wall pier caused an error condition.
	12204	An incident affecting composite beam design was resolved. When a slender section was assigned to some composite beams and a design code prohibiting its use was selected, the Composite Beam Design Summary and Composite Beam Design Details tables were left empty. Design reports were unaffected. This incident affected all versions of ETABS capable of designing composite beams. Two related enhancements to the Composite Beam Design Summary and Composite Beam Design Details tables were implemented: 1.) they now include two columns possibly reporting up to two applicable design warnings 2.) when the shear studs on a beam are not uniformly distributed, they are called out with the same formatting as in the display of design results, the design reports, and the output to DXF and EXR files.

Design – Composite Beam Incidents Resolved

*	Ticket	Description
	12140	An incident affecting composite beam design per the Eurocode and IS11384 codes was resolved. When a beam was designed without enough shear studs on it to develop the mandated minimum percentage of composite action, ETABS computed its superimposed dead load and live load deflections per Section 3 of SCI NCCI: Modified Limitations on Partial Shear Connection in Beams for Building PN002a-GB, as documented, instead of computing them based on the moment of inertia of the steel section, resulting in an underestimate. Note that this incident had no impact on the strength of any of the designed beams. This incident affected all versions of ETABS capable of designing composite beams per the Eurocode and IS11384 codes.
	12150	Two incidents affecting composite beam design per the AISC 360-16 and AISC 360-22 codes were resolved. 1.) For beams longer than 30 ft., the minimum percentage of composite action displayed in the Composite Beam Design Overwrites form did not show the minimum specified by AISC Commentary Section I3.2d. This was a display problem only and the AISC specified minimum was applied by ETABS during design. Only ETABS v23.2.0 was affected. 2.) An excessive number of shear studs was specified for some beams when all of the following conditions happened simultaneously: a.) The Consider Axial Forces option was selected for the beam b.), the forces for some of the load combinations selected for strength checks were small enough to be resisted by the steel beam alone c.) the maximum bending moments for these load combinations occurred near one of the beam ends. No unsafe design was selected and turning off the Consider Axial Forces option or removing from consideration the load combinations with these smaller forces resolved the issue. This incident affected all versions of ETABS capable of taking axial forces into account when designing composite beams.
	12178	An incident affecting composite beam design per the AISC 360-10, AISC 360-16, AISC 360-22 and KDS 14 31 80 - 2022 codes was resolved. The shear stud capacity of composite beams supporting a solid slab on both sides was computed per the AISC 360-05 formula, instead of the applicable formula, resulting in an overestimate. All versions of ETABS capable of designing composite beams per the AISC 360-10, AISC 360-16, AISC 360-22 or KDS 14 31 80 - 2022 code were affected.

Design – Composite Column Incidents Resolved

*	Ticket	Description
	11980	An incident has been resolved for the AISC 360-22 composite column design in which the interpolation of the value K_c was incorrect. It is now corrected and the design manual is also updated.

Design – Concrete Frame Incidents Resolved

*	Ticket	Description
	12041	An incident has been resolved for the Eurocode 2-2004, EN 1992-1-1:2023, and Italian NTC 2008 concrete frame designs in which the results for crack width design for beam members were inconsistent with multi-threads used for design. Columns were unaffected.
	12068	An incident has been resolved to avoid an error condition when a model used auto select lists for concrete frame design, but some frames were either not concrete or were set to not be designed.
	12070	An incident was resolved for KDS 14 20 00:2022 code where eta and beta1 values were incorrect when model default database units were not in N-mm units for concrete and PT frame and slab design.

*	Ticket	Description
	12092	An incident was resolved for KDS 14 20 00:2022, KBC 2016 and KBC 2009 concrete frame design codes where minimum eccentricity for column design was not correctly enforced. It was using 1.5mm plus 0.03*column dimension instead of 15mm plus 0.03*column dimension.
	12130	An incident has been resolved for the EC 1992-1-2:2023 concrete frame design in which an error was produced. The error only occurred for members with end-length offsets and in-span loadings that resulted in a difference in shear between two consecutive design stations being not greater than five percent (5%) of the maximum shear along the entire length of the member.
	12131	An incident has been resolved for the EN 1992-1-1:2023 concrete frame design in which the design of the columns having section with reinforcement to be checked could not be done.
	12135	An incident has been resolved in concrete frame design code IS 456-2000 in which the beam flexural rebar calculation was not correct if the beam was assigned a beam type section but had a large compressive axial load. There was no issue if the beam had zero axial load or was in tension. For compressive axial loads below balanced point with the flag set to ignore axial load if beneficial (default setting) there was no issue either. Also, there was no issue if the beam had been assigned a column type section.
	12180	An incident was resolved for KDS 14 20 00 : 2022 concrete frame design code where Avmin was not enforced when $\Phi \cdot V_c / 2 < V_u < \Phi \cdot V_c$ for beam shear design.

Design – Concrete Shell

Incidents Resolved

*	Ticket	Description
	12190	An incident has been resolved for the Eurocode 2-2004 concrete shell design in which the value of Cot(Theta) in the Preference could not be set larger than 1.0. Now the upper limit is not imposed on it in the form. But if the input value is greater than the upper limit, Cot(Theta) will be taken as the upper limit during the design. In addition, the issue with export and re-importing the model text file causing the loss of the concrete shell design requests has also been resolved.
	12202	An incident was resolved for IS 456:2000 shear wall concrete capacity when Hw/Lw was greater than 1 and wall. For Pier with Hw/Lw > 1, shear capacity was computed using equation (b) of section 23.4.3 instead of computing shear capacity using the lesser of equation (a) and (b) of Section 23.4.3.

Design – Shear Wall

Incidents Resolved

*	Ticket	Description
	12010	An incident was resolved for ACI 318-19 and ACI 318-25 shear wall design where "Identify Shear Failure" and "Identify All Failures" were unable to show "OS" message when pier was failing in shear unless wall results were viewed.

Design – Steel Frame

Incidents Resolved

*	Ticket	Description
	11987	An incident has been resolved for the EN 1993-1-1:2022 steel frame design in which classification of the web for combined axial and bending about ZZ-axis was overly conservative. In addition, details of the classification for pipe section were not shown properly in the design report but the design results of the pipe section were not affected.
	12043	An incident has been resolved in steel frame design code "Chinese 2018" in which the second term of the interaction equation GB50017-2017 5.5.7-2 was being used as $(M_x^{II}) / (\phi_b \cdot \gamma_x \cdot W_x \cdot f)$ instead of $(M_x^{II}) / (\phi_b \cdot W_x \cdot f)$ in the program. This affects only the members with no axial force or with compression, and when the design check was performed based on DAM (Direct analyses Method). The tension members were being checked with correct equation. The previous implementation was slightly unconservative.

*	Ticket	Description
	12071	An incident has been resolved for the EN 1993-1-1:2005/A1:2014 steel frame design in which the C_{my} factor is calculated incorrectly for the case using the maximum member deflection in Table A.2 of the Annex A.
	12121	An incident has been resolved for the EN 1993-1-1:2005, EN 1993-1-1:2022, NTC2008, and NTC2018 steel frame designs in which the square root of sum of squares (SRSS) rule has been removed from calculating the demand/capacity combination ratio. The SRSS is not specified to be used in any of the mentioned design codes.
	12123	An incident affecting interactive steel design of members with slenderness ratios larger than allowed by the selected steel design code was resolved. Clicking on the Report... button in the Interactive Steel Design and Review form while the message about the excessive slenderness was selected in the Strength Checks caused an abnormal termination. This incident affected ETABS v23.2.0 only. A workaround was to select any other check in the Strength Checks prior to clicking the Report... button.
	12181	An incident was resolved to fix a unit-conversion issue in the Beam-to-Column Shear Minor Axis connection that caused inaccurate eccentricity calculations when using metric units.

External Import and Export

Incidents Resolved

*	Ticket	Description
	12128	An incident affecting re-import of composite beam design preferences previously exported to Microsoft Access was resolved. Several fields with a dot in their names, such as "Min. Long. Spacing" were not imported. When that happened, default values for these preferences were used by ETABS and design results agreed with the model. This incident affected all versions of ETABS capable of exporting models to Microsoft Access. Export followed by import back to and from Excel or XML files were not affected.
	12165	An incident was resolved for ASCE 7-22 auto seismic load pattern where parameter "Site Class" and "Long-Period Transition Period" were set to default values when *.Set/*.e2k text file was imported.

Graphics

Incidents Resolved

*	Ticket	Description
	12142	An incident was resolved which was affecting the drafting of joints in elevation view when offsets were applied in the DirectX mode.

Loading

Incidents Resolved

*	Ticket	Description
	12084	An incident was resolved for ASCE 7-22 auto wind load where user has specified a single step wind load using user defined wind pressure coefficient. Wind load was causing change in results when model was rerun when all of these conditions were met.
	12161	An incident was resolved for ASCE 7-22 auto wind load where Wind Direction angle was not saved for "Include Frame Objects (Open Structure)" option when form was closed.
	12176	An incident was resolved for auto seismic KDS 41 17 00 : 2022 code where enforcement of upper limit for time period could be incorrect when time period was program computed and S_{d1} values were in the following range: $0.3 \leq S_{d1} < 0.4$ and $0.15 < S_{d1} < 0.2$

Results Display and Output

Incidents Resolved

*	Ticket	Description
	12045	An incident was resolved for ASCE 7-22 Wind load report which was not updating the Winward and Leeward Pressure for rectangular building and some parameters were not correctly updated to ASCE 7-22 code. These were only reporting issues and wind design results were not affected.

*	Ticket	Description
	12056	An incident was resolved where the "Modal Load Participation Ratios" database table may not be available in models that included area springs. Additionally, the table output was improved so that loads from line springs are correctly labeled.
	12184	An incident was resolved for IS 456:2000 concrete frame design code where beam shear rebar in "Diagram" output for concrete frame design was showing incorrect shear rebar. This was just a display issue and design was not affected.

Structural Model

Incidents Resolved

*	Ticket	Description
	12090	An incident was resolved where Shear V2 hinges that did not include both BC and CD points in the backbone curve of the hinge property (Define menu > Section Properties > Frame/Wall Nonlinear Hinges) and were assigned as a Vertical Wall Hinge (Assign menu > Shell > Wall Hinge) could have behavior in the positive direction that did not follow the backbone curve. This issue only occurred for Vertical Wall Hinges and only occurred when hinges were modeled as separate link elements (Analyze menu > Analysis Model for Nonlinear Hinges).

User Interface

Incidents Resolved

*	Ticket	Description
	11996	An incident was resolved to display correct moment connection when only one end is released.
	12082	An incident was resolved to correctly display line thickness on screen in regular graphics mode.
	12083	An incident was resolved where the visibility of selected frames (using dashed lines) were not obvious in the DirectX mode.